	TABLE OF BEAM SIZES AND DESIGN DATA (4 FT. MAX. BEAM SPACING)								H-20
General Notes	BEAM SDAN	ROLLED BEAM		DEFLECTION IN INCHES		UNFACTORED BEAM END REACTION			
SPECIFICATIONS: All references to the standard Specifications are to the		BEAM MEMBER	BEAM DEPTH (IN.)	STEEL ONLY	TOTAL DL	DC (kips)	DW (kips)	LL+I (kips)	DESIGN LUAD
current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, with current supplemental specifications	15' Max.	W16x40	16.0	0.00	0.06	5.39	0.45	25.96	CLEARANCEAND
All references to the AASHTO Specifications are to the current edition of the	!	W14x43	13.7	0.01	0.07	5.42	1		LOADLANG WIDTH 101-01
AASHTO LRFD Bridge Design Specifications, with interims.	!	W12x50	12.2	0.01	0.08	5.48	1		(3.0 m)
DESIGN LOADS : Beam sections are designed for H20 Live Load.		W10x60	10.2	0.01	0.08	5.56	1		
The fatigue truck is also set at H2U.	20' Max.	W18x50	18.0	0.01	0.12	7.17	0.60	27.45	
DESIGN LOAD DISTRIBUTION : Contrary to AASHTO LRFD Bridge Design Specification, the design moment and shear distribution for all beams is 0.6 lanes.	!	W16x57	16.4	0.01	0.12	7.25	1		
	!	W14x61	13.9	0.02	0.15	7.29			(4.8 m) (1.8 m
FUTURE WEARING SURFACE: These beams are designed for a 15 PSF future wearing	!	W12x65	12.1	0.02	0.18	7.34	1		8,000 005 3 6,000 005 (0.6 m) (35 kN) (145 kN)
surface load.		W10x77	10.6	0.03	0.21	7.47]		
SUBSTRUCTURE DESIGN LOADS: Unfactored design reaction forces per beam end.	25' Max.	W21x55	20.8	0.02	0.20	8.94	0.75	28.34	
DC (kips): Beam, Slab, and assumed Type III railing dead loads. DW (kips): Future wearing surface. LL+I (kips): LL with Dynamic load allowance.	!	W18x65	18.4	0.02	0.21	9.07			These beams were sized according
	!	W16x67	16.3	0.03	0.24	9.10			meet federal minimum design HL-93
	!	W14x74	14.2	0.04	0.29	9.19			nor KY minimum KYHL-93.
MATERIAL DESIGN SPECIFICATIONS:	!	W12x79	12.4	0.05	0.35	9.26			Use of these beams will limit the loaded
for Steel Reinforcement $FY = 60000 PSI$	30' Max.	W21x73	21.2	0.04	0.30	10.93	0.90	28.94	size of trucks allowed to cross the bridge,
for Class "AA" Deck Concrete $F'C = 4000 PSI$!	W18x76	18.2	0.05	0.36	10.98			
	!	W16x77	16.5	0.06	0.43	11.00			roads with little potential for future
MATERIAL STEEL A.S.T.M AASHTO	!	W14x90	14.0	0.07	0.49	11.20			development. These beams can
High Strength Low Alloy *A709 GR 50 *M270 GR 50	!	W12x96	12.7	0.09	0.59	11.30			not suitable for a loaded gravel truck
Structural Steel	35' Max.	W24x94	24.3	0.05	0.34	13.07	1.05	29.37	or concrete truck. They are not
	!	W21x101	21.4	0.06	0.38	13.20			with construction, agriculture, or coal.
High strength bolts, nuts, and washers F3125 Grade A325 M-164 Type 1	!	W18x97	18.6	0.08	0.52	13.13			
Sheet lead and Pig lead B29-79	ļ!	W14x109	14.3	0.12	0.75	13.35			
All steel in longitudinal rolled wide flange beams shall meet the longitudinal Charpy V-Notch toughness test for non-fracture critical components Zone 2 in accordance with the following:	40' Max.	W24x104	24.1	0.08	0.51	15.09	1.20	29.69	
	!	W21x111	21.5	0.10	0.60	15.23			
M270 GR 50 (up to 2" thickness) of 15 ft-lbs at 40°F.	!	W18x130	19.3	0.12	0.66	15.63			
	!	W14x145	14.8	0.19	0.97	15.94			
Sampling and testing procedures shall be in accordance with AASHTO T243 current edition, utilizing (H) frequency testing. When plate thickness exceeds $1^{1/2}$ ", frequency of testing shall be (P).	45' Max.	W27x102	27.1	0.11	0.70	16.87	1.35	29.93	
	!	W24x104	24.1	0.13	0.81	16.92	-		
	!	W21x122	21.7	0.15	0.87	17.34	4		
HIGH STRENGTH BOLT CONNECTIONS: Unless otherwise specified on the plans, all	!	W18x143	19.5	0.19	0.96	17.83	4		
nuts, and washers. Open holes shall be ¹³ /a ⁶ diameter. Type I galvanized bolts, shall be used as described in AASHTO M164. All high strength bolted field connections are to be installed with "direct tension indicators" (DTI's) in accordance with the Standard Specifications and ASTM F959. All DTI's shall be manufactured from a steel conforming		W14x159	15.0	0.30	1.42	18.20	<u> </u>		
	50' Max.	W30x116	30.0	0.13	0./9	19.06	1.50	30.13	
	!	W27x129	27.6	0.15	0.83	19.40	-		
to the chemical requirements of ASTM A325 for Type I galvanized steel. UT's snall be installed under the bolt head with the bumps facing the underside of the bolt head. Put	!	W24x131	24.5	0.18	0.99	19.45	-		
a hardened washer under the nut and tension from the nut.	!	W21x14/	22.1	0.22	1.12	19.86	-		
BEVELED EDGES : Bevel all exposed edges ¾".		W18x1/5	20.0	0.27	1.22	20.58	1.05	20.20	
REINFORCEMENT : Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of	55' Max.	W33X130	33.1	0.10	0.87	21.32	1.05	30.29	
	!	W 30X 148	30.7	0.18	0.89	21.83	-		
reinforcement. All steel reinforcement is to be epoxy coated in accordance	!	W2/X140	27.4	0.21	1.05	21.//	-		
with Section 811.10 of the Specifications.	!	W24X140	24./	0.20	1.30	21.//	-		KENTUCKY
CORRECTION: These heaves and all steal companents are to be bet din	COL Max	W21X100	22.5	0.31	1.42	22.34	1.00	- 10 42	DEPARTMENT OF HIGHWAYS
galvanized. If hot dip galvanizing is cost prohibitive then all steel	60 Max.	W33X152	33.3	0.21	1.04	23.90	1.80	30.43	
components must be painted. Unpainted weathering steel is not recommended within 10 foot of moving water. Additionally these beams do not meet	!	W 30X 1/ 3	30.4	0.24	1.00	24.54	-		nzu Sieel
fatigue design requirements of unpainted weathering steel.	!	W2/X101	27.0	0.29	1.50	24.17	-		BEAM
	!	W24X170	23.2	0.55	1.55	24.05	-		STANDARDS
BRIDGE DECK: A galvanized steel metal grid deck may be substituted for the 8"		WZIXZUI	23.0	0.42	1.05	23.40	<u> </u>		STANDARD DRAWING NO BSB-001
concrete deck. This will improve the load rating, but smaller beams are not to be used.									Batachen 02-26-20
									DIRECTOR DIVISION OF STRUCTURAL DESIGN DATE
									APPROVED